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**Exam I**

Friday April 4, 2014

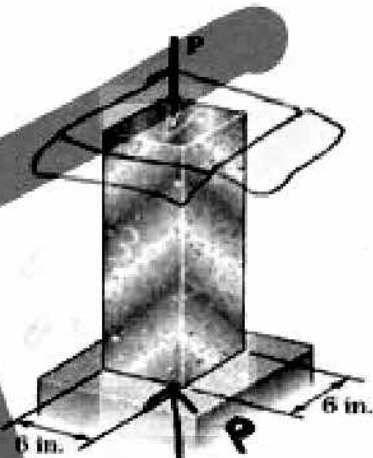
Duration: 1h 30min

All work must be shown to receive full credit.

**Problem 1.** (20 pts.) A centric load  $P$  is applied to the aluminum block shown below. The ultimate normal and shearing stresses of aluminum are  $\sigma_u = 29$  ksi and  $\tau_u = 7.25$  ksi. The modulus of elasticity is  $E = 9.57 \times 10^6$  psi and poisson's ratio  $\nu = 0.33$ .

- a - Calculate the maximum allowable load  $P$  that can be applied using a safety factor of 3 ? (12 pts.)
- b - If the load in (a) is applied, calculate the edge length of the deformed square cross-sectional area which was originally 6 in. (8 pts.)

$\sigma_u = 29 \text{ ksi}$   
 $\tau_u = 7.25 \text{ ksi}$   
 $E = 9.57 \times 10^6 \text{ psi}$   
 $\nu = 0.33$   
 $n = 3$



a)  $\sigma_{all} = \frac{\sigma_u}{n} = \frac{29}{3} = 9.6667 \text{ ksi}$

$\sigma_{all} = \frac{P}{A} \Rightarrow P = \sigma_{all} \times A$

$P = 9.6667 \times 10^3 \times (6 \times 6)$

$P = 348 \times 10^3 \text{ lb} \quad (C)$

$\epsilon_{all} = \frac{3.25 \times 10^{-3}}{3} = 2416.6667 \text{ psi}$

$\delta = \epsilon_{all} \times A \times 2$   
 $= 2416.6667 \times 6 \times 6$   
 $= 87000.0012 \text{ lb}$

$P_{max} = 87000.0012 \text{ lb}$

b+

$$\Sigma x = \frac{F x}{E}$$

$$\Delta x = \frac{87000,0011}{6 \times 6}$$

$$\Delta x = 2416,667 \text{ psi}$$

$$\Sigma x = \frac{2416,667}{9,57 \times 10^4} = 2,525 \times 10^{-4}$$

(C)  
3/2  
Hans  
down

$$\nu = -\frac{\Sigma y}{\Sigma x}$$

$$\Sigma y = -\nu \Sigma x$$

$$\Sigma y = -0,33 \times (-2,525 \times 10^{-4})$$

$$\Sigma y = 8,3325 \times 10^{-5}$$

$$\Sigma y = \frac{\Delta \text{edge}}{\text{edge}} \rightarrow \Delta \text{edge} = 8,3325 \times 10^{-5} \times 6$$

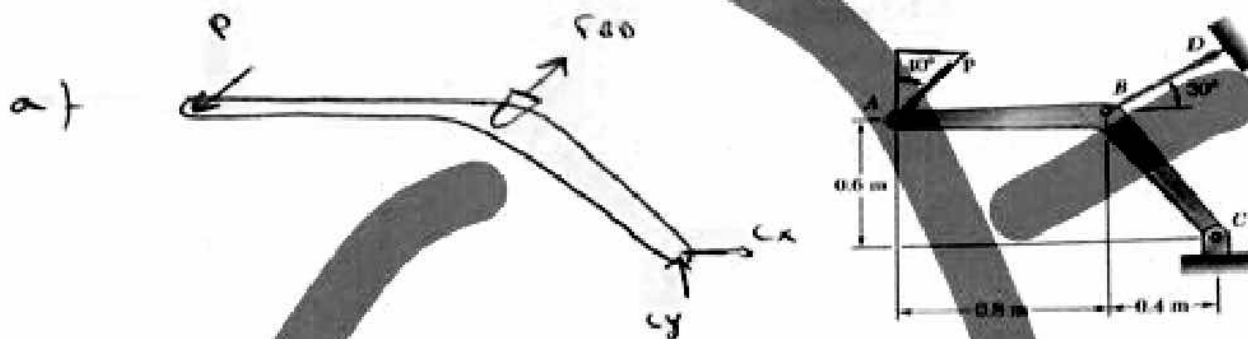
$$= 4,9995 \times 10^{-4}$$

$$\approx 5 \times 10^{-4} \text{ in}$$

$$\text{New edge} = 5 \times 10^{-4} + 6 = 6,0005 \text{ in}$$

Problem 2. (20 pts.) Member ABC is supported by a steel pin and bracket (double-shear) at C and a 0.5 m long steel cable BD with a 2 cm diameter. Assume that for steel,  $E = 200$  GPa,  $\tau_u = 100$  MPa and  $\sigma_u = 200$  MPa. If the applied load  $P = 20$  kN:

- Calculate the normal stress in the cable. (10 pts.)
- Calculate the elongation in the cable. (4 pts.)
- Determine the minimum required pin diameter using a safety factor of 2. (6 pts.)



$$\sum M_C = 0;$$

$$P \sin 40 \times 0.6 + P \cos 40 \times 1.2 - F_{C0} \cos 30 \times 0.6 - F_B \sin 30 \times 0.4 = 0$$

$$P(\sin 40 \times 0.6 + \cos 40 \times 1.2) = F_{C0} (0.6 \cos 30 + 0.4 \sin 30)$$

$$P \times 1.305 = F_{C0} \times 0.72$$

$$F_{C0} = \frac{20 \times 10^3 \times 1.305}{0.72} = 36250 \text{ N.}$$

$$\sigma_{C0} = \frac{F_{C0}}{A_{C0}} = \frac{36250}{\frac{\pi}{4} (2 \times 10^{-2})^2} = \frac{36250}{3.1416 \times 10^{-4}} = 115.38 \text{ MPa}$$

$$b) \quad \epsilon_{C0} = \frac{F_{C0} L_{C0}}{A_{C0} E_{C0}} = \frac{36250 \times 0.5}{\frac{\pi}{4} (2 \times 10^{-2})^2 \times 200 \times 10^9}$$

$$= \frac{18125}{62831853.07} = 2.884683 \times 10^{-4} \text{ m}$$

c) shear in pipe

$$\tau_{\text{all}} = \frac{F_{\text{all}}}{A} = \frac{100}{2} = 50 \text{ MPa}$$

$$\rightarrow \Sigma F_x = 0$$

$$-P \sin 40 + F_{00} \cos 30 - C_x = 0$$

$$C_x = F_{00} \cos 30 - P \sin 40$$

$$C_x = 36250 \times \cos 30 - 20000 \sin 40$$

$$C_x = 18537,67 \text{ N} \quad (\rightarrow)$$

$$\uparrow \Sigma F_y = 0$$

$$-P \cos 40 + F_{00} \sin 30 + C_y = 0$$

$$-C_y = F_{00} \sin 30 - P \cos 40$$

$$C_y = P \cos 40 - F_{00} \sin 30$$

$$C_y = 20000 \cos 40 - 36250 \times \sin 30$$

$$C_y = -2806,1113 \text{ N} \quad (\leftarrow \text{is opposite reaction})$$

~~component~~ ↓ downward

$$\tau_{\text{all}} = 50 \text{ MPa} = \frac{F_{\text{R0}}}{A}$$

$$F_{\text{R0}} = \sqrt{C_x^2 + C_y^2}$$

$$= \sqrt{343645703 + 7963039,274}$$

$$= 18748,5532 \text{ N} \quad (\searrow \text{R0})$$

$$A = \frac{F_{\text{R0}}}{\tau_{\text{all}}} = \frac{18748,5532}{50 \times 10^6} = 3,75 \times 10^{-4} \text{ m}^2$$
$$\frac{\pi}{4} d^2 = 3,75 \times 10^{-4}$$

$$d_{\min} > 0,02185 \text{ m}$$

~~if the pin will fail it will be caused by the shear stress~~

$$e_{\text{all}} = \frac{F_{\text{oc}}}{A_{\text{pin}}} \Rightarrow A_{\text{pin}} = \frac{F}{e} = \frac{18748,5532}{2 \times 50 \times 10^6}$$

$A_{\text{pin}}$

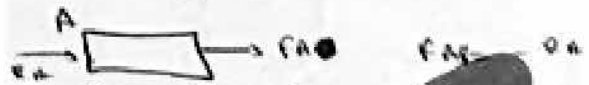
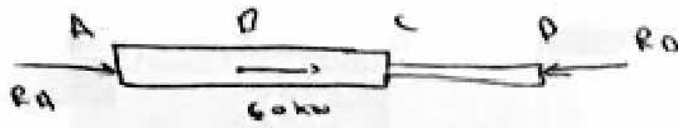
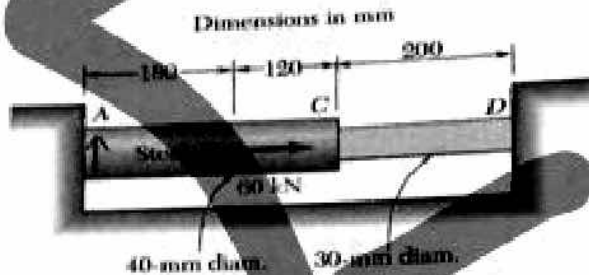
$$= 1,8748 \times 10^{-4} \text{ m}^2$$

$$A = \frac{\pi}{4} d^2$$

$$d = 0,01545 \text{ m} \Rightarrow \text{reject}$$

**Problem 3.** (20 pts.) Two cylindrical rods made of steel are joined at C and restrained by rigid supports at A and D. If for the steel used  $E = 200 \text{ GPa}$ , calculate:

- a - The maximum normal stress. (14 pts.)
- b - The maximum shear stress. (6 pts.)



$$\sum F_x = 0 \Rightarrow RA + 60 \times 10^3 - RE = 0$$

$$\delta_{AD} = 0$$

$$\delta_{AC} + \delta_{CD} = 0$$

$$\left(\frac{PL}{AE}\right)_{AC} + \left(\frac{PL}{AE}\right)_{CD} = 0$$

$$\frac{-RA \times 180 \times 10^{-3}}{\frac{\pi}{4} (40 \times 10^{-3})^2 \times 200 \times 10^9} + \frac{(-RA - 60 \times 10^3) \times 200 \times 10^{-3}}{\frac{\pi}{4} (30 \times 10^{-3})^2 \times 200 \times 10^9} = 0$$

$$-112500 RA + 75000 (-RA - 60 \times 10^3) + (-RA - 60 \times 10^3) \times 222222,222 = 0$$

$$-403722,222 RA - 45000000 \times 10^3 - 13333333,333 = 0$$

$$RA = -43525,42372 \text{ N}$$

$$\Rightarrow RE = RA + 60 \times 10^3$$

$$RE = 16474,57628 \text{ N}$$

← opposite direction

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$$a) \sigma_a = \frac{F_a}{A_a} = \frac{435254372}{\frac{\pi}{4} (40 \times 10^{-3})^2} = 47,766 \text{ MPa}$$

$$\sigma_a = \frac{435254372}{\frac{\pi}{4} (40 \times 10^{-3})^2} = 47,766 \text{ MPa}$$

$$\sigma_b = \frac{16474152}{\frac{\pi}{4} (70 \times 10^{-3})^2} = 23,306 \text{ MPa}$$

$$\sigma_{max} = 47,766 \text{ MPa}$$

$$b) \tau_{AB} = \frac{435254372}{(40 \times 10^{-3}) \times 10^{-6}} = \frac{F}{A}$$

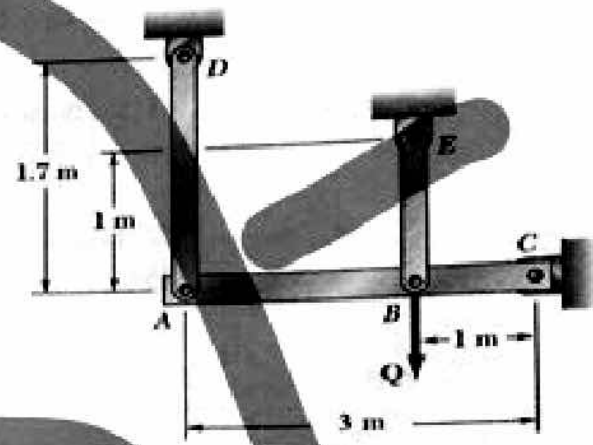
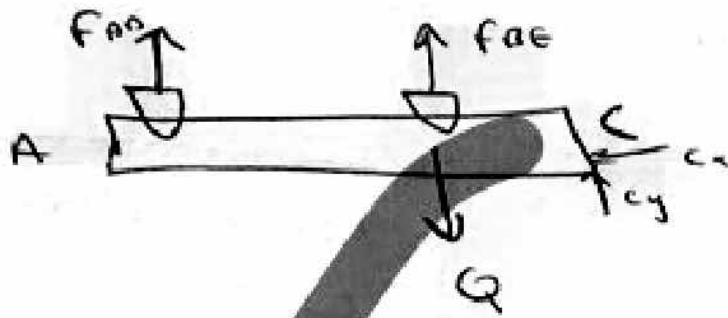
$$= 6045133,211 \text{ Pa}$$

$$\tau_B = \frac{60 \times 10^3}{(120 \times 40) \times 10^{-6}} = 12500000 \text{ Pa}$$

$$\tau_B = \frac{16474152}{(200 \times 70) \times 10^{-6}} = 2745761,667 \text{ Pa}$$

**Problem 4.** (20 pts.) The rigid bar ABC is supported by two links, AD and BE with  $2.25 \times 10^{-4} \text{ m}^2$  cross section and made of steel ( $E = 200 \text{ GPa}$ ). If  $Q = 100 \text{ kN}$ , determine:

- a - The normal stress in each of the links AD and BE. (14 pts.)
- b - The maximum deflection of point B. (6 pts.)



$\delta_C = 0$

$$\sum \mathcal{M}_C = 0 \Rightarrow -F_{AD} \times 3 - F_{BE} \times 1 + Q \times 1 = 0$$

$$3F_{AD} + F_{BE} = 100 \times 10^3$$

$$\frac{\delta_B}{\delta_A} = \frac{1}{3} \Rightarrow \delta_B = \frac{\delta_A}{3}$$

$$\delta_B = \frac{F_{BE} L_{BE}}{A_{BE} E_{BE}}$$

$$F_{BE} = \frac{\delta_A}{3} \times \frac{A_{BE} \times E_{BE}}{L_{BE}} = 15000000 \delta_A$$

$$\delta_A = \frac{F_{AD} L_{AD}}{A_{AD} \times E_{AD}} \Rightarrow F_{AD} = \frac{\delta_A \times A_{AD} \times E_{AD}}{L_{AD}}$$

$$= 26470586.24 \delta_A$$

$$\Rightarrow 3F_{AD} + F_{BE} = 100 \times 10^3$$



$$79411764,72 \Delta a + 15000000 \Delta a = 1000000$$

$$94411764,72 \Delta a = 1000000$$

$$\Delta a = 1,06 \times 10^{-3} \text{ m}$$

$$\Rightarrow F_{AD} = 280588,23 \text{ N (T)}$$

$$\Rightarrow F_{DE} = 15300 \text{ N (T)}$$

$$\sigma_{AD} = \frac{F_{AD}}{A} = \frac{280588,23}{2,25 \times 10^{-4}} = 1247,051100 \text{ MPa}$$

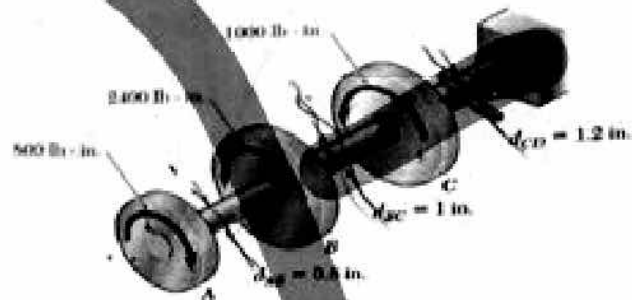
$$\sigma_{DE} = \frac{15300}{2,25 \times 10^{-4}} = 70,667 \text{ MPa}$$

$$\frac{\delta a}{\Delta a} = \frac{1}{3}$$

$$\delta a = \frac{\Delta a}{3} = \frac{1,06 \times 10^{-3}}{3} = 3,533 \times 10^{-4} \text{ m}$$

**Problem 6.** (20 pts.) The length of shafts AB, BC, and CD are  $L_{AB} = 2$  m,  $L_{BC} = 2.5$  m and  $L_{CD} = 4$  m, respectively. If the shafts are made of steel with  $E = 29 \times 10^6$  psi and  $G = 11.2 \times 10^6$  psi, determine:

- The maximum shear stress in the shaft system. (8 pts.)
- The angle of twist of B relative to C. (6 pts.)
- The angle of rotation at A. (6 pts.)



a)  $\tau$

$$\tau_A = \frac{T r}{J}$$

$$J_{AB} = \frac{\pi}{32} C^4 = \frac{\pi}{32} \left( \frac{0.5}{2} \right)^4 = 0.040212 \text{ in}^4$$

$$J_{BC} = \frac{\pi}{32} C^4 = \frac{\pi}{32} (0.5)^4 = 0.09817477 \text{ in}^4$$

$$J_{CD} = 0.20357 \text{ in}^4$$

$$\tau_A = \frac{800 \times 0.4}{0.040212} = 7957.82 \text{ psi}$$

$$\tau_B = \frac{(-2400 + 800) \times 0.5}{0.09817477} = -8148.733 \text{ psi}$$

$$\tau_C = \frac{(-2400 - 1000 + 800) \times 0.6}{0.20357} = -7663.2116 \text{ psi}$$

$$|\tau_{max}| = |\tau_D| = 8148.733 \text{ psi}$$

bt

$$\omega_0 = \frac{T_0 L_0}{J_0 G_0}$$

$$= \frac{1000 \times 4 \times 2,07}{20357 \times 16,2 \times 10^6}$$

$$= 2,43 \text{ rad}$$

$\omega_c = \omega_0 + \omega_c$

$$= 2,43 + \frac{2400 \times 2,5 \times 2,07}{20357 \times 16,2 \times 10^6}$$

$$= 2,43 + 0,0138$$

$$= 2,4438 \text{ rad}$$

connect into in

bt

$$\omega_0 = \frac{T_0 L_0}{J_0 G_0}$$

$$= \frac{1000 \times 4}{20357 \times 16,2 \times 10^6} = (-1,7544 \times 10^{-3} \text{ rad})$$

$\omega_c = \omega_0 + \omega_c$

$$= 1,7544 \times 10^{-3} + \frac{2400 \times 2,5}{20357 \times 16,2 \times 10^6}$$

$$= 1,7544 \times 10^{-3} + 5,8567 \times 10^{-3}$$

$$= 7,2111 \times 10^{-3} \text{ rad}$$

$$\phi_B = \frac{T_0 L_0}{J_0 G_0} = \frac{-2400 \times 2.5}{9.0587672 \times 10^6 \text{ rad}}$$

$$\phi_B = -5.425 \times 10^{-3} \text{ rad}$$

relative to C  
considering  $\phi_C = \phi_B + \phi_C$   
total

$$C + \phi_A = \phi_A + \phi_B + \phi_C \quad \phi_C = \phi_B$$

$$\phi_A = \frac{T_0 L_A}{J_0 G_0} + (-5.425 \times 10^{-3}) + (-1.7564 \times 10^{-3})$$

$$\phi_A = \frac{800 \times 2}{9.040212 \times 10^6} - 5.425 \times 10^{-3} - 1.7564 \times 10^{-3}$$

$$= 3.5255 \times 10^{-3} - 5.425 \times 10^{-3} - 1.7564 \times 10^{-3}$$

$$= -3.6539 \times 10^{-3} \text{ rad}$$

(deformation in same direction as torques applied on B & C)